

STRUCTURAL DESIGN - II

02. Bolted Connections

Kiran S R

Lecturer

Department of Civil Engineering

Central Polytechnic College Thiruvananthapuram

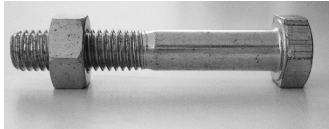
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Connections in Steel Structures

Types of Connections: Connections are of the following types:

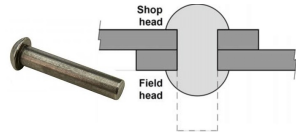
- BOLTS
- WELDS
- RIVETS



BOLT



WELD



RIVET

Bolted Connections

Types of Bolts

Bolts are available in the following types:

- 1 Black bolts/ Unfinished Bolts/ C-grade bolts
- 2 Turned Bolts/ Close Tolerance Bolts
- 3 Ribbed bolts
- 4 High strength bolts

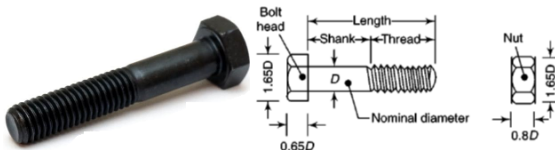


Bolted Connections

Types of Bolts

1) Black bolts/ Unfinished Bolts/ C-grade bolts

- Ordinary, Unfinished and Least expensive bolts.
- Generally used in structures subjected to static loads; not recommended if subjected to impact, fatigue or dynamic loads.
- Here, slip occurs between connected plates, when force is applied. As a result, the plates bear against the bolts. Such type of connections are known as "Bearing-type connections".
- Made of mild steel rods with square or hexagonal heads & nuts.
- Designated by its Diameter (i.e., Shank Diameter in mm) as M5 to M36.
- Cross-sectional area at threads = $0.78 \times$ Cross-sectional area at shank
- Generally used grade(or property class) = 4.6 grade bolts
 - Here, 4 represents $(1/100)$ th of f_{ub}
 - 0.6 represents $\frac{f_{yb}}{f_{ub}}$
where f_{yb} & f_{ub} represent yield strength & ultimate strength of bolt.



Types of Bolts

2) Turned bolts/ Close Tolerance bolts

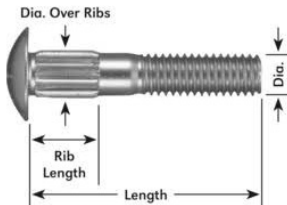
- Similar to unfinished bolts, but shanks of these bolts are formed from a hexagonal rod.
- Surface of these bolts are prepared carefully to fit in the hole. Hence, they permit no slippage between connected parts.
- They are mainly used under dynamic loads.

3) Ribbed bolts

- Have rounded head and raised ribs parallel to shank.
- While driving into the hole, the ribs cut the edges around the hole and produce a tight fit.



TURNED BOLT



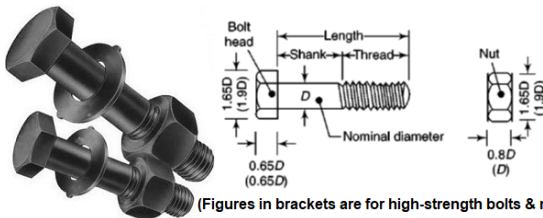
RIBBED BOLT

Bolted Connections

Types of Bolts

4) High Strength bolts

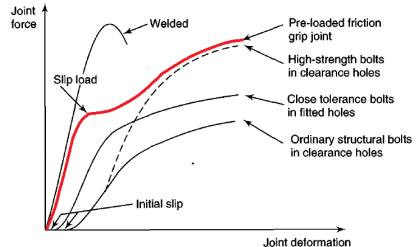
- Made from bars of medium carbon steel.
- Less ductile than black bolts. Bolt material do not have a well-defined yield point. Instead of yield stress, proof stress ($= 0.7 \times \text{Ultimate strength of bolt}$) is used.
- Bolts of sizes M16 to M36 are available. Grades (or property class) available are 8.8S, 10.9 S etc. Letter S denotes high-strength structural bolts.
- Here, slip between connected plates is prevented by applying initial pretension (to bolts) using torque wrenches, which induces friction. Such high-strength bolts are called High Strength Friction Grip (HSFG) bolts. Such connections are known as Non-slip connections or Friction-type connections.
- These are very expensive. The material cost of HSFG bolts are about 50% higher than black bolts and require special workmanship for installation.



Types of Bolts

Advantages of HSFG bolts over black bolts:

- No slip between connected elements, thus providing rigid connections.
- Due to the clamping action, load is transmitted by friction only and the bolts are not subjected to shear and bearing.
- Since the load is transferred by friction, there is no stress concentration in the holes.
- Due to the smaller number of bolts, the gusset plate sizes are reduced.
- Joint Deformation is minimized (see fig.)
- Noiseless fabrication, since the bolts are tightened with wrenches.

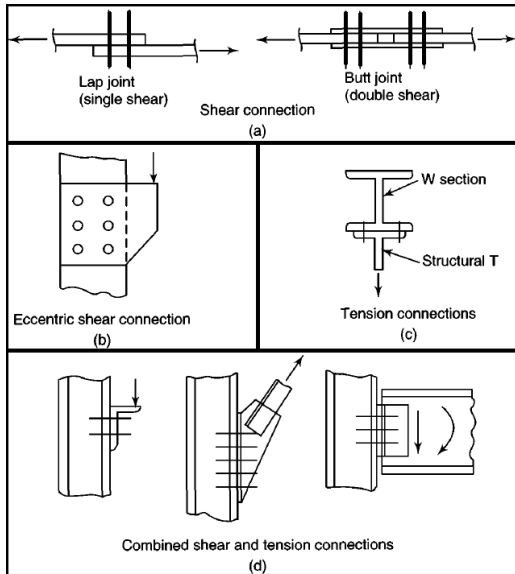


Advantages of Bolts over Riveted or Welded connections

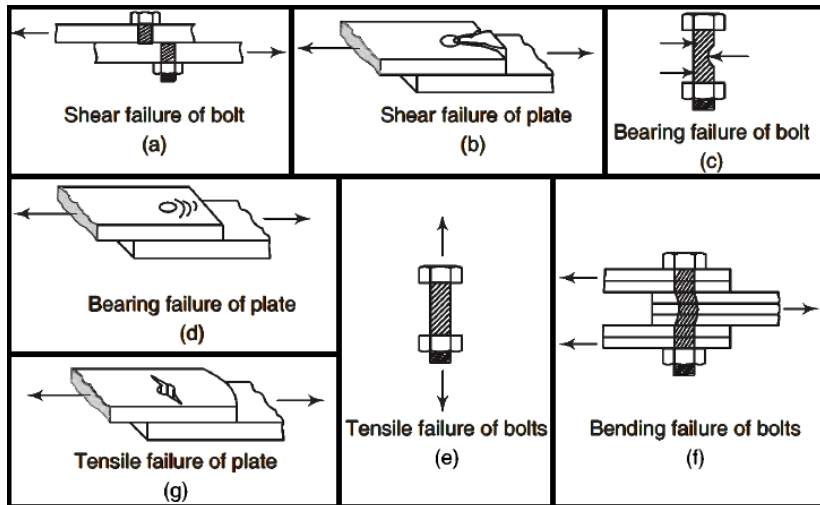
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Bolted Connections

Types of Bolted connections



Probable modes of failure of Bolted connections



Important Terminologies & Relevant provisions of IS800

= Diameter of Bolt at the shank region

= d + clearance (Cl.10.2.1 & Table19, Page73)

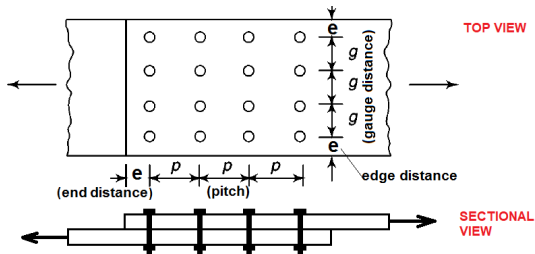
Bolted Connections

Important Terminologies & Relevant provisions of IS800

3) Pitch (p): (Cl.10.2.2 & 10.2.3, Page74)

Centre-to-centre distance between two adjacent rows of bolts, measured in the direction of application of force.

- Min. $p = 2.5 d$
- Max. $p = 32t$ or 300mm, whichever is less (generally)
 - = 16t or 200mm, whichever is less (tension members)
 - = 12t or 200mm, whichever is less (compression members)where t is the thickness of the thinner plate in mm



Important Terminologies & Relevant provisions of IS800

Centre-to-center distance between bolts measured perpendicular to the direction of application of force.

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- The diagram illustrates the nomenclature of a bolted joint. It consists of two views: a TOP VIEW and a SECTIONAL VIEW.
- TOP VIEW:** Shows a rectangular plate with four bolts arranged in a 2x2 grid. The end distance (e) is the distance from the edge to the center of the first bolt. The pitch (p) is the distance between the centers of adjacent bolts. The gauge distance (g) is the distance between the centers of bolts in the same row.
- SECTIONAL VIEW:** Shows the bolts passing through the plate. The end distance (e) and pitch (p) are labeled.

Bolted Connections

Important Terminologies & Relevant provisions of IS800

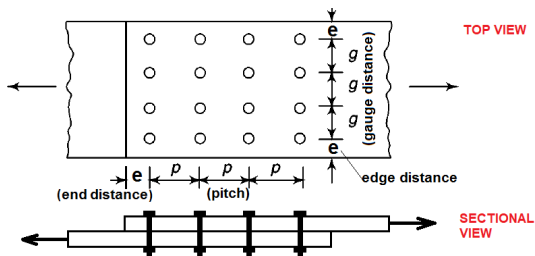
5) End distance (e): (Cl.10.2.4, Page74)

Distance measured in the direction of stress from the centre of a hole to the end of the element.

6) Edge distance (e): (Cl.10.2.4, Page74)

Distance measured at right angles to the direction of stress from the centre of a hole to the adjacent edge.

- Min. $e = 1.7d_o$ (for hand-flame cut), $1.5d_o$ (for machine-flame cut)
- Max. $e = 12\epsilon t$, where $\epsilon = \sqrt{(250/f_y)}$



Bolted Connections

Important Terminologies & Relevant provisions of IS800

7) Partial Safety factor for bolts:

(Page30)

Table 5 Partial Safety Factor for Materials, γ_m
(Clause 5.4.1)

Sl No.	Definition	Partial Safety Factor	
i)	Resistance, governed by yielding, γ_{m0}	1.10	
ii)	Resistance of member to buckling, γ_{m0}	1.10	
iii)	Resistance, governed by ultimate stress, γ_{m1}	1.25	
iv)	Resistance of connection:	<i>Shop Fabrications</i>	<i>Field Fabrications</i>
a)	Bolts-Friction Type, γ_{mf}	1.25	1.25
b)	Bolts-Bearing Type, γ_{mb}	1.25	1.25
c)	Rivets, γ_{mv}	1.25	1.25
d)	Welds, γ_{mw}	1.25	1.50



Bolted Connections

Design Strength of Ordinary Black Bolts

The design strength of Black bolts (or BOLT VALUE) is the least of the following:

- Design Shear capacity of bolts (V_{dsb})
- Design Bearing capacity of bolts (V_{dpb})
- Design Tension capacity of bolts (T_{db})



Design Strength of Ordinary Black Bolts

Design Shear capacity of bolts (V_{dsb})

10.3.3 Shear Capacity of Bolt Cl.10.3.3 - Page75

The design strength of the bolt, V_{dsb} as governed shear strength is given by:

$$V_{dsb} = V_{nsb} / \gamma_{mb}$$

where V_{nsb} = nominal shear capacity of a bolt, calculated as follows:

$$V_{nsb} = \frac{f_u}{\sqrt{3}} (n_a A_{nb} + n_s A_{sb})$$

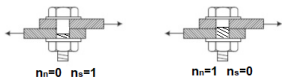
f_u = ultimate tensile strength of a bolt;

n_a = number of shear planes with threads intercepting the shear plane;

n_s = number of shear planes without threads intercepting the shear plane;

A_{sb} = nominal plain shank area of the bolt; and

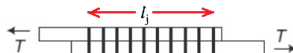
A_{nb} = net shear area of the bolt at threads, may be taken as the area corresponding to root diameter at the thread.



The Design shear capacity of Bolt V_{dsb} shall be reduced in case of Long joints, larger Grip lengths and usage of packing plates. Their reducing factors are given below:

10.3.3.1 Long joints

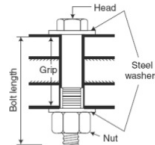
When the length of the joint, l_j (that is the distance between the first and last rows of bolts in the joint, measured in the direction of the load transfer) exceeds $15d$ in the direction of load, the nominal shear capacity (see 10.3.2), V_{nb} shall be reduced by the factor β_{lj} , given by: $\beta_{lj} = 1.075 - l_j / (200 d)$ but $0.75 \leq \beta_{lj} \leq 1.0$ where d = Nominal diameter of the fastener.



10.3.3.2 Large grip lengths

When the grip length, l_g (equal to the total thickness of the connected plates) exceeds 5 times the diameter of the bolts, the design shear capacity shall be reduced by a factor β_{lg} , given by

$$\beta_{lg} = 8 d / (3 d + l_g) = 8 / (3 + l_g / d)$$



10.3.3.3 Packing plates

The design shear capacity of bolts carrying shear through a packing plate in excess of 6 mm shall be decreased by a factor, β_{pk} given by: $\beta_{pk} = (1 - 0.0125 t_{pk})$ where t_{pk} = thickness of the thicker packing, in mm.



Design Bearing capacity of bolts (V_{dpb})

10.3.4 Bearing Capacity of the Bolt

The design bearing strength of a bolt on any plate, V_{dpb} as governed by bearing is given by:

$$V_{\text{dpb}} = V_{\text{npb}} / \gamma_{\text{mb}}$$

where

$$V_{npb} = \text{nominal bearing strength of a bolt}$$

$$= 2.5 k_b d t f_u$$

where

$$k_b \text{ is smaller of } \frac{e}{3d_0}, \frac{p}{3d_0} - 0.25, \frac{f_{ub}}{f_u}, 1.0;$$

e, p = end and pitch distances of the fastener along bearing direction;

d_0 = diameter of the hole;

f_{ub}, f_u = ultimate tensile stress of the bolt and the ultimate tensile stress of the plate, respectively;

d = nominal diameter of the bolt; and

t = summation of the thicknesses of the connected plates experiencing bearing stress in the same direction, or if the bolts are countersunk, the thickness of the plate minus one half of the depth of countersinking.

Design Tension capacity of bolts (T_{db})

10.3.5 Tension Capacity

A bolt subjected to a factored tensile force, T_b shall satisfy:

$$T_b \leq T_{db}$$

CI.10.3.5
Page76

where

$$T_{\text{db}} = T_{\text{gb}} / \gamma_{\text{mb}}$$

T_{nb} = nominal tensile capacity of the bolt,
calculated as:

$$0.90 f_{ub} A_n < f_{yb} A_{sb} (\gamma_{mb} / \gamma_{m0})$$

where

 f_{ub} = ultimate tensile stress of the bolt, f_{yb} = yield stress of the bolt,

A_n = net tensile stress area as specified in the appropriate Indian Standard (for bolts where the tensile stress area is not defined, A_n shall be taken as the area at the bottom of the threads), and

A_{sh} = shank area of the bolt.

